Colusa Unified School District

COLUSA HIGH SCHOOL AG BARN SITE UTILITIES

Bid Package #16-100

ADDENDUM NO. 1

March 4, 2016

Owner:	Colusa Unified School District 8408 Watt Avenue Colusa, CA 95843
Architect:	Architectural Nexus 1900 3rd Street, Suite 500 Sacramento, CA 95811
Project Manager:	Capital Program Management, Inc. 1851 Heritage Lane, Suite 210 Sacramento, CA 95815

This Addendum has been prepared to clarify, modify, delete, or add to the drawings and/or specifications for the above referenced project, and revisions to items listed here shall supersede description thereof prior to the above stated date. All conditions not specifically referenced here shall remain the same. It is the obligation of the Prime Contractor to make subcontractors aware of any items herein that may affect submitted bids.

Acknowledge receipt of this addendum by inserting its number and date in the bidding documents. Failure to do so may subject bidder to disqualification.

All addenda items refer to the plans and specifications unless specifically noted otherwise.

TOTAL PAGES IN THIS ADDENDUM (including attachments): **38**

PART A - BIDDING AND CONTRACT REQUIREMENTS

1.1 **The bid date has been changed**. The new bid date is: **no later than Thursday, March 17, 2016 at 1:30:00 p.m.** at the Colusa Unified School District – District Office, 745 10th Street, Colusa, CA 95932.

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1.2 Section 00 8000, Section 1.03, Scope of Work, "Excluded" add the following:
"4. Initial Compaction testing only."

1.3 Section 00 8000, Section 3.02, Schedules, Preliminary Construction Schedule, replace as follows:

Preliminary Construction Schedule

Schedule the Work to accommodate the following milestone requirements:

- Post Bid Document Phase Milestone **(Start Date, Not Later than 3/28/16)**
  - Notice of Intent to Award
  - Post-Bid Submittals
  - Project Submittals/Shop Drawings
  - Post Bid Document Phase Milestone **(Completion Date, Not Later than 4/8/16)**

- Zero Float Phase Milestone **(Start Date: approximately 4/11/16)**
  - Notice to Proceed
  - Submission of all remaining required submittals in accordance with 00 7200
  - Mobilization/initial layout
  - Site Work improvements

- Remaining Scope of Work Phase Milestone **(Start Date: Not Later than 4/25/16)**
  - AC Paving Patch
  - Punchlist development and completion
  - Remaini9ng Scope of Work Phase Milestone **(Completion Date: Not Later than 6/10/16)**

1.4 Section 00 8000, Section 3.03, Liquidated Damages and **REPLACE:**
"The amount of liquidated damages is indicated below (also refer to General Conditions Section 00700, Article 28):" **with**
"The amount of liquidated damages is indicated below (also refer to General Conditions Section 00 7200, Article 28):"

1.5 Refer to Section 00 4113 – Bid Form, Item A: "All associated work in the Contract Documents enumerated in Article 2 of the Agreement Form specific to Colusa High School" **REPLACE with**: "All associated work in the Contract Documents enumerated in Article **3** of the Agreement Form specific to Colusa High School".

1.6 Refer to Section 01 3300, Section 1.02 and **DELETE:** A. Section 01 1216 Phasing of the work.

1.7 Refer to Section 01 3300, Section 3.01.A.2: **REPLACE with** "Confirmed that no substitutions have been included. If substitutions are included, CONTRACTOR shall eliminate them from the

submittal and process them in accordance with Section 00 7200 General Conditions Article 30. Materials."

1.8 Refer to Section 01 7329, Section 1.02: **REPLACE with**
A. Section 00 7200 - General Conditions.
B. Section 00 8000 - Special Provisions.

1.9 Refer to Section 01 7329, Section 1.03.D: **REPLACE with**
"Submit all materials to be used in cutting and patching in accordance with Specification Section 00 7200."

1.10 Refer to Section 01 7329, Section 2.01.B: **REPLACE with**
"Product Substitution: For any proposed change in materials, submit request for substitution in accordance with Specification Section 00 7200."

1.11 Refer to Section 01 7329, Section 2.01.B: **REPLACE with**
"Product Substitution: For any proposed change in materials, submit request for substitution in accordance with Specification Section 00 7200."

PART B - TECHNICAL REQUIREMENTS

1.12 Insert "*Geotechnical Engineering Report*" from Wallace Kuhl and Associates Inc. dated December 18, 2015. (31 pages)

1.13 Refer to Section 26 00 10, Basic Electrical Requirement, Section 1.1.C.5: and **DELETE**:
"Refer to Division 05, Miscellaneous Metals."

1.14 Refer to Section 26 00 10, Basic Electrical Requirement, Section 1.1.C.6: and **DELETE**:
"Refer to Division 06, Rough Carpentry."

1.15 Refer to Section 26 00 10, Basic Electrical Requirement, Section 1.1.C.5: and **DELETE**:
"Refer to Division 07, Thermal and Moisture Protection."

1.16 Refer to Section 26 00 10, Basic Electrical Requirement, Section 1.1.C.5: and **DELETE**:
"Refer to Division 08, Access Doors also, Division 05, Metals."

1.17 Refer to Section 26 00 10, Basic Electrical Requirement, Section 1.1.C.5: and **DELETE**:
"Refer to Division 09, Painting."

1.18 Refer to Section 26 00 10, Basic Electrical Requirement, Section 1.1.C.5: and **DELETE**:
"Refer to Division 09, Acoustical Treatment."

1.19 Refer to Section 26 05 53, Electrical Identification, Section 1.1.B.1: and **DELETE**:
"Division 09: Painting."

Colusa Unified School District

COLUSA HIGH SCHOOL AG BARN SITE UTILITIES

Bid Package #16-100

ADDENDUM NO. 1

PART C - DRAWINGS

1.20 Drawing C1.1, Utility Plan notes: add attached detail 4/C1.1 to note 21. Sewer connection.

List of Attachments

1.21 Bid Conference Agenda dated February 17, 2016 (1 page)

1.22 Pre-Bid Sign-in Sheet dated February 17, 2016 (2 pages)

End of Addendum

Wallace Kuhl & Associates

Geotechnical Engineering Report

COLUSA HIGH SCHOOL AG BARN BUILDING

WKA No. 10796.01P

December 18, 2015

Prepared For:

Architectural Nexus, Inc.

1990 Third Street, Suite 500

Sacramento, California 95811

Geotechnical Engineering Report

COLUSA HIGH SCHOOL AG BARN BUILDING

Colusa, California

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Geotechnical Engineering Report
COLUSA HIGH SCHOOL AG BARN BUILDING
Colusa, California
WKA No. 10796.01P
December 18, 2015

INTRODUCTION

We have completed a geotechnical engineering investigation for the proposed Colusa High School Ag Barn Building project located at 901 Colus Avenue in Colusa, California. The purposes of our work have been to explore the existing site, soil, and groundwater conditions across the site, and to provide geotechnical conclusions and recommendations for the design and construction of the proposed improvements. This report represents the results of our work.

Scope of Work

Our scope of work has included the following tasks:

1. site reconnaissance;
2. review of aerial photographs;
3. subsurface exploration, including the hand augering and sampling of two borings to depths of approximately nine feet below existing site grades;
4. laboratory testing of select soil samples;
5. engineering analysis; and,
6. preparation of this report.

Our evaluation was performed in general accordance our *Geotechnical Engineering Services Proposal,* dated November 9, 2015.

Figures and Attachments

A Vicinity Map showing the location of the site is included as Figure 1. A Site Plan showing the approximate locations of the hand augers and various site details is included as Figure 2. The Logs of Soil Borings are presented as Figures 3 and 4. An explanation of the symbols and classification system used on the logs appears on Figure 5. Appendix A contains general information regarding project concepts, exploratory methods used during our field investigation, and laboratory test results not included on the boring logs.

Proposed Development

We understand the project will consist of the design and construction of a single-story, slab-on-grade, pre-engineered building. We understand the new building will be about 60 feet by 100 feet in plan area and will house animal pens. Associated improvements will consist of pavements to support farming equipment.

FINDINGS

Site Description

The subject site is located in the southeast corner of the Colusa High School campus located at 901 Colus Avenue in Colusa, California. The project site is located on an existing sand and gravel drive area that is being used for onsite parking. At the time of our field exploration, the project area contained a light growth of surface vegetation and was visibly clear of any structures. Farming equipment consisting of an agricultural loader and attached plow was observed on site.

Topography of the site is essentially flat with an average surface elevation of about +53 feet relative to mean sea level (msl), based on review of the United States Geological Survey (USGS) *Topographic Map of the Colusa Quadrangle, California*, dated 1991.

Surface and Subsurface Soil Conditions

Two exploratory hand auger borings were performed on December 7, 2015 at the approximate locations indicated on the attached Site Plan (Figure 2). The soil conditions at the boring locations consisted of roughly 1½ feet of fill containing silty sand and gravel as well as asphalt and concrete pieces. The subgrade soils beneath the fill consisted primarily of clayey silt and silty clay with variable amounts of sand to the explored depths of about nine feet below the ground surface.

For soil conditions at a specific location, please refer to the Logs of Soil Borings provided on Figures 3 and 4.

Groundwater

Groundwater was not encountered within our hand auger borings performed on December 7, 2015, which extended to depths of approximately nine feet below existing site grades.

To supplement the groundwater information obtained from the field exploration, we reviewed available California Department of Water Resources (DWR) records for wells in the vicinity of the project site. DWR monitored well identified as # 16N02W25B002M located approximately one mile northeast of the project site, which has been monitored by the DWR from February 1966 to October 2015. The ground elevation at the well is indicated to be approximately +55 feet msl. Groundwater measurements obtained from the well indicate a "high" groundwater elevation of approximately +51½ feet msl (about 3½ feet below existing grades at the well) occurred in March 1998, and a "low" groundwater elevation of approximately +23½ feet msl (about 31½ feet below existing grades at the well) occurred in August 2015. Measurements taken in the last 10 to 15 years have shown groundwater levels at the well location to vary between approximately six and 31½ feet below existing grades.

CONCLUSIONS

Seismic Code Design

Section 1613 of the 2013 edition of the California Building Code (CBC) references the American Society of Civil Engineers (ASCE) Standard 7-10 for seismic design. The following seismic parameters provided in Table 1 were determined based on the site latitude and longitude using the public domain computer program developed by the USGS. The seismic design parameters summarized in Table 1 may be used for seismic design of the proposed commercial development.

TABLE 1
2013 CBC/ASCE 7-10 SEISMIC DESIGN PARAMETERS

Latitude: 39.1996° N Longitude: 122.0188° W	ASCE 7-10 Table/Figure	2013 CBC Table/Figure	Factor/ Coefficient	Value
0.2-second Period MCE	Figure 22-1	Figure 1613.3.1(1)	S_S	0.812 g
1.0-second Period MCE	Figure 22-2	Figure 1613.3.1(2)	S_1	0.336 g
Soil Class	Table 20.3-1	Section 1613.3.2	Site Class	D
Site Coefficient	Table 11.4-1	Table 1613.3.3(1)	F_a	1.175 g
Site Coefficient	Table 11.4-2	Table 1613.3.3(2)	F_v	1.728 g
Adjusted MCE Spectral Response Parameters	Equation 11.4-1	Equation 16-37	S_{MS}	0.954 g
	Equation 11.4-2	Equation 16-38	S_{M1}	0.581 g
Design Spectral Acceleration Parameters	Equation 11.4-3	Equation 16-39	S_{DS}	0.636 g
	Equation 11.4-4	Equation 16-40	S_{D1}	0.387 g

TABLE 1 2013 CBC/ASCE 7-10 SEISMIC DESIGN PARAMETERS				
Seismic Design Category	Table 11.6-1	Section 1613.3.5(1)	Risk Category I to IV	D
	Table 11.6-2	Section 1613.3.5(2)	Risk Category I to IV	D

Notes: MCE = Maximum Considered Earthquake
g = gravity

A liquefaction analysis was not performed at the site. However, the soil conditions encountered at the boring locations performed for this evaluation, previous evaluations performed in the vicinity of the site, and our experience in the Colusa area indicate the site likely is underlain by low to medium plasticity silts and clays, which are typically not susceptible to liquefaction. Therefore, it is our opinion that the potential for liquefaction of the soils beneath the site is considered to be very low.

Soil Expansion Potential

Laboratory test results on near-surface clays and silts indicate these materials are low plasticity soils when tested in accordance with ASTM D4318 test method (see Figure A2) and possess a moderate expansion potential when tested in accordance with ASTM D4829 test method (see Figure A4). Based on the laboratory test results and our local experience, we anticipate the native silts and clays will not exert significant expansion pressures on building foundations, interior floor slabs and exterior flatwork provided the recommendations of this report are followed.

Bearing Capacity

Our work indicates that undisturbed native soils, processed and compacted native soils, and engineered fills constructed in accordance with the recommendations of this report will be capable of supporting the proposed improvements.

We estimate total settlement for shallow footing foundations using the recommended maximum net allowable bearing pressure presented below, should be less than one inch. Differential settlements are estimated to be about one-half the total settlement. These settlement estimates are based on the available boring information, our experience with similar structures and soil conditions, and field verification of suitable bearing soils during foundation construction.

Excavation Conditions

Based on the information obtained at the boring locations and our local experience, we anticipate the soils at the site will be readily excavatable with conventional earthmoving and trenching equipment.

Subsurface remnants from previous development of the site (i.e., foundations, underground tanks, vaults, etc.) as well as surface fill material may be encountered at the site and can be slow to excavate with a standard, rubber-tired backhoe; however, experience has shown that excavators can remove these materials with moderate effort.

We anticipate soils exposed in trench sidewalls and below-grade excavations will consist of interbedded layers of clayey silt and silty clay with variable amounts of sand. Based on the soil conditions encountered at the borings, excavations associated with building foundations, shallow trenches for utilities, and other excavations less than five feet deep should stand vertically for short periods of time required for construction (i.e. less than one 1 day), unless cohesionless, saturated or disturbed soils are encountered. These unstable conditions may result in caving or sloughing; therefore, the contractor should be prepared to brace or shore the excavations, if necessary. Excavations deeper than five feet should be sloped or braced in accordance with current Occupational Safety and Health Administration (OSHA) requirements regulations.

Temporarily sloped excavations should be constructed no steeper than a 1½ horizontal to one vertical (1½:1) inclination. Temporary slopes likely will stand at this inclination for the short-term duration of construction, provided significant pockets of loose and/or saturated granular soils are not encountered that could slough into excavations. Flatter slopes would be required if these conditions are encountered.

The contractor must provide a safely sloped excavation or an adequately constructed and braced shoring system in accordance with federal, state and local safety regulations for individuals working in an excavation that may expose them to the danger of moving ground. If material is stored or heavy equipment is operated near an excavation, proper shoring must be used to resist the extra pressure due to the superimposed loads.

Soil Suitability for Use in Fill Construction

The on-site native soils and existing fill encountered in our borings are considered suitable for use in engineered fill construction, provided these materials are free of significant organics,

rubble, and other deleterious materials, and are at moisture contents capable of achieving the desired degree of compaction. Imported materials, if necessary, ideally should be granular soils possessing an Expansion Index of less than 50, a maximum three-inch particle size, and be approved by our office prior to importing the materials to the site.

Pavement Subgrade Quality

The subgrade soil beneath the on-site fill soils consists primarily of clayey silt and silty clay with variable amounts of sand. These near-surface clays are relatively poor quality materials for support of asphalt concrete pavements and will require thicker pavement sections to compensate for the lower strength of the soils. A Resistance ("R") value of 5 has been used in the design of pavements for the subject site.

Groundwater

Based on current explorations performed at the site and historical groundwater data, we anticipate excavations greater than five feet below existing site grades may encounter groundwater and require dewatering (depending on the time of year). For design purposes, groundwater should be anticipated at a depth of five feet below the ground surface. If groundwater is encountered, the use of sumps, submersible pumps, deep wells or a well point system could be used as methods to lower the groundwater level. The dewatering method used will depend on the soil conditions, depth of the excavation and amount of groundwater present within the excavation. Dewatering, if required, should be the contractor's responsibility. The dewatering system should be designed and constructed by a dewatering contractor with local experience. We recommend the selected dewatering system lower the groundwater level to at least two feet below the bottom of the proposed excavations.

Seasonal Water

Infiltrating surface run-off water from seasonal moisture during the winter and spring months will create saturated surface soil conditions. It is probable that grading operations attempted following the onset of winter rains and prior to prolonged drying periods will be hampered by high soil moisture contents. Such soils, intended for use as engineered fill, will require a prolonged period of dry weather and aeration or chemical treatment to reach a moisture content suitable for proper compaction.

Soil Corrosion Potential

A sample of near-surface was submitted to Sunland Analytical Lab for testing to determine pH, chloride and sulfate concentration, and minimum resistivity to help evaluate the potential for corrosive attack upon buried concrete. The results of the corrosivity testing are summarized in Table 2 and a copy of the analytical test report is presented in Figure A5.

TABLE 2
SOIL CORROSIVITY TESTING

Analyte	Test Method	Bulk B1 (0 - 3')
pH	CA DOT 643 Modified*	7.97
Minimum Resistivity	CA DOT 643 Modified*	1,230 Π-cm
Chloride	CA DOT 422	68.3 ppm
Sulfate	CA DOT 417	72.1 ppm
	ASTM D516	63.65 mg/kg

Notes: * = Small cell method; Ω-cm = Ohm-centimeters; ppm = Parts per million

The California Department of Transportation Corrosion and Structural Concrete Field Investigation Branch 2012, Corrosion Guidelines (Version 2.0), considers a site to be corrosive to foundation elements if one or more of the following conditions exists for the representative soil and/or water samples taken: has a chloride concentration greater than or equal to 500 ppm, sulfate concentration greater than or equal to 2000 ppm, or the pH is 5.5 or less. Based on this criterion, the on-site soil is not considered corrosive to steel reinforcement properly embedded within Portland cement concrete (PCC) for the samples tested.

Table 4.2.1 – *Exposure Categories and Classes*, American Concrete Institute (ACI) 318-11, Section 4.2, as referenced in Section 1904.1 of the 2013 CBC, indicates the severity of sulfate exposure for the sample tested is *Not Applicable*. Ordinary Type I-II Portland cement is considered suitable for use on this project, assuming a minimum concrete cover as detailed in ACI 318-11 Section 7.7 is maintained over the reinforcement.

Wallace-Kuhl & Associates are not corrosion engineers. Therefore, if it is desired to further define the soil corrosion potential at the site a corrosion engineer should be consulted.

RECOMMENDATIONS

The recommendations presented below are appropriate for typical construction in the late spring through fall months. The on-site soils likely will be saturated by rainfall in the winter and early spring months, and will not be compactable without drying by aeration or chemical treatment to dry the soils. Should the construction schedule require work during wet conditions, additional recommendations can be provided, as conditions dictate.

Site preparation should be accomplished in accordance with the provisions of this report. A representative of the Geotechnical Engineer should be present during site grading to evaluate compliance with our recommendations and the approved project plans and specifications. The Geotechnical Engineer of Record referenced herein should be considered the Geotechnical Engineer that is retained to provide geotechnical engineering observation and testing services during construction.

Site Clearing and Subgrade Preparation

Initially, the site should be cleared of existing structures, including below-grade structures (if any), debris, and other deleterious materials. Where practical, the clearing should extend a minimum of five feet beyond the limits of the proposed structural areas of the site.

Existing underground utilities to be abandoned within the proposed building pads or new pavement areas should be completely removed and/or rerouted as necessary. Utilities located outside the building areas should be properly abandoned (i.e., fully grouted provided the abandoned utility is situated at least 2½ feet below the final subgrade level to reduce the potential for localized "hard spots"). Depressions resulting from removal of underground structures (e.g., foundations, utilities, etc.) should be cleaned of loose soil and properly backfilled in accordance with the recommendations of this report.

Following site clearing activities, surface fill soils should be sub-excavated from all structural areas of the site (i.e. building pad, exterior flatwork, pavement areas, etc.) to expose undisturbed native soils (anticipated to be at a depth of about 18 inches below the ground surface). Any debris exposed by the required sub-excavation should be removed. The native soils exposed following the recommended sub-excavation, as well as any other surfaces to receive fill, achieved by excavation or remain at grade, should be scarified to a depth of at least six inches, thoroughly moisture conditioned to at least two percent above the optimum moisture content, and compacted to at least 90 percent relative compaction within building pad and exterior flatwork areas and to at least 95 percent relative compaction within pavement areas.

Relative compaction should be based on the maximum dry density as determined in accordance with the ASTM D1557 Test Method.

Compaction operations should be performed in the presence of the Geotechnical Engineer's representative who will evaluate the performance of the subgrade under compactive load and identify loose or unstable soils that could require additional subgrade preparation.

Engineered Fill Construction

All fill placed within the construction area should be an approved material, free of significant quantities of organics or other deleterious materials. Onsite native soils and native soils from the immediate vicinity of the site used for engineered fill should be moisture conditioned to at least two percent over the optimum moisture content and maintained in that condition. The fill should be spread in level layers not exceeding nine (9) inches in loose thickness and compacted to a minimum of 90 percent of the maximum dry density. Maximum dry densities shall be determined in accordance with ASTM D1557.

Imported fill should be an approved compactable granular material, have an Expansion Index of 50 or less and be free of particles larger than three (3) inches in maximum dimension. The contractor also should supply appropriate documentation for imported fill materials indicating the materials are free of known contamination and have corrosion characteristics within acceptable limits. The Geotechnical Engineer's must approve import material before being transported to the project site.

Utility Trench Backfill

Bedding and initial backfill for utility construction should conform with the pipe manufacturers recommendations and applicable sections of the governing agency standards. General trench backfill should consist of engineered fill backfilled in maximum nine-inch thick loose lifts with each lift compacted to at least 90 percent of the maximum dry density. Utility trench backfill within the upper six inches of the final subgrade within pavement areas should be compacted to at least 95 percent of the maximum dry density.

We recommend that all underground utility trenches aligned nearly parallel with foundations be at least five feet from the foundations, wherever possible. If this is not practical, the trenches should not encroach on a zone extending at a one horizontal to one vertical (1:1) inclination below the foundations.

Foundation Design

The proposed building may be supported upon continuous and/or isolated spread foundations embedded at least 18 inches below lowest adjacent soil grade. Lowest adjacent soil grade should be measured from the surface on which the capillary break gravel is placed or exterior compacted soil subgrade, whichever is lower. Continuous foundations should maintain a minimum width of 12 inches and isolated spread foundations should be at least 18 inches in plan dimension. Foundations so established may be sized for maximum net allowable soil bearing pressures of 2500 pounds per square foot (psf) for dead plus live loads, with a one-third increase for total loads including the short-term effects of wind or seismic forces. The weight of the foundation concrete extending below lowest adjacent soil grade may be disregarded in sizing computations.

We recommend that all foundations be reinforced to provide structural continuity, mitigate cracking and permit spanning of local soil irregularities. The structural engineer should determine final foundation reinforcing requirements.

Resistance to lateral foundation displacement for conventional foundations may be computed using an allowable friction factor of 0.25, which may be multiplied by the effective vertical load on each foundation. Additional lateral resistance may be computed using an allowable passive earth pressure of 250 psf per foot of depth. These two modes of resistance should not be added unless the frictional value is reduced by 50 percent since full mobilization of these resistances typically occurs at different degrees of horizontal movement.

We recommend that all foundation excavations be observed by the Geotechnical Engineer's representative prior to placement of reinforcement and concrete to verify firm bearing materials are exposed.

Interior Floor Slab Support

Interior concrete slab-on-grade floors can be supported upon the soil subgrade prepared in accordance with the recommendations in this report and maintained in that condition (two percent over optimum moisture content). We recommend that interior floor slabs be reinforced to provide structural continuity, mitigate cracking and permit spanning of local soil irregularities. The structural engineer should determine final floor slab reinforcing requirements.

Floor slabs should be at least four inches thick and underlain by a layer of free-draining crushed rock, serving as a deterrent to migration of capillary moisture. The crushed rock layer should be at least four inches thick and graded such that 100 percent passes a one-inch sieve and no

appreciable amount passes a No. 4 sieve. Additional moisture protection may be provided by placing a vapor retarder membrane (at least 10-mils thick) directly over the crushed rock. The membrane should meet or exceed the minimum specifications as outlined in ASTM E1745, and be installed in strict conformance with the manufacturer's recommendations.

For increased support for heavily loaded slab-on-grade floors or areas subjected to equipment traffic, slabs may be underlain by at least six inches of Class 2 aggregate base compacted to 95 percent of the maximum dry density as determined by ASTM D1557 test method. A durable vapor barrier could be placed over the aggregate base. Slab thickness and reinforcement should be determined by the structural engineer based on anticipated slab loads.

Floor slab construction over the past 30 years or more has included placement of a thin layer of sand or pea gravel over the vapor retarder membrane. The intent of the sand or pea gravel is to aid in the proper curing of the slab concrete. However, recent debate over excessive moisture vapor emissions from floor slabs includes concern for water trapped within the sand or pea gravel. As a consequence, we consider the use of the sand or pea gravel as optional. The concrete curing benefits should be weighed against efforts to reduce slab moisture vapor transmission.

The recommendations presented above are intended to mitigate any significant soils-related cracking of the slab-on-grade floors. More important to the performance and appearance of a Portland cement concrete slab is the quality of the concrete, the workmanship of the concrete contractor, the curing techniques utilized, and the spacing of control joints.

Floor Slab Moisture Penetration Resistance

It is likely the floor slab subgrade soils will become saturated at some time during the life of the structure, especially when slabs are constructed during the wet season and when constantly wet ground or poor drainage conditions exist adjacent to structures. For this reason, it should be assumed that all interior slabs, particularly those intended for moisture-sensitive floor coverings or materials, require protection against moisture or moisture vapor penetration. Standard practice includes placing a layer of rock and a vapor retarder membrane (and possibly a layer of sand) as discussed above. Recommendations contained in this report concerning foundation and floor slab design are presented as minimum requirements only from the geotechnical engineering standpoint.

Use of sub-slab gravel and a vapor retarder membrane will not "moisture proof" the slab, nor does it assure that slab moisture vapor transmission levels will be low enough to prevent

damage to floor coverings or other building components. It is emphasized that we are not slab moisture proofing or moisture protection experts. The sub-slab gravel and vapor retarder membrane simply offer a first line of defense against soil-related moisture. If increased protection against moisture vapor penetration of the slab is desired, a concrete moisture protection specialist should be consulted. It is commonly accepted that maintaining the lowest practical water-cement ratio in the slab concrete is one of the most effective ways to reduce future moisture vapor penetration of the completed slab.

Exterior Concrete Flatwork

Areas to receive exterior flatwork should be scarified, thoroughly moisture conditioned, and properly compacted, as recommended in the Site Clearing and Subgrade Preparation and Engineered Fill Construction sections of this report immediately prior to placement of imported fill (if any) and concrete.

Sidewalks and other concrete flatwork should be placed on at least six inches of aggregate base compacted to at least 90 percent relative compaction over the prepared soil subgrade. The subgrade should be uniformly moisture conditioned to at least two percent above the optimum moisture content and compacted to at least 90 percent of the maximum dry density just prior to concrete placement.

Flatwork should be at least four inches thick and reinforced for crack control. For crack control, reinforcement should include, as a minimum, chaired No. 3 rebar located on maximum 24-inch centers, both ways, throughout slabs. Accurate and consistent location of the reinforcement at mid-slab is essential to its performance and the risk of uncontrolled drying shrinkage slab cracking is increased if the reinforcement is not properly located within the slab.

Exterior flatwork should be constructed independent of the building foundations. Isolated column foundations should be structurally separated from adjacent flatwork by the placement of a layer of felt, or other appropriate material, between the flatwork and foundations. Practices recommended by the Portland Cement Association (PCA) for proper placement and curing of concrete should be followed during exterior concrete flatwork construction.

The architect or civil engineer should determine the final thickness, strength, reinforcement, and joint spacing of exterior slab-on-grade concrete.

Pavement Design

Pavement design analysis has been performed based upon the procedures contained in the *California Highway Design Manual*, 6th Edition, using Traffic Indices (TI's) considered appropriate for the anticipated traffic conditions. Our subsurface exploration indicates the subgrade soils are poor quality for the support of asphalt concrete pavements and we have used an R-value of five in our the pavement design. We can provide additional pavement sections for other TI's as necessary.

TABLE 3
PAVEMENT DESIGN ALTERNATIVES (R-value = 5)

Traffic Condition	Traffic Index	Type B Asphalt Concrete (inches)	Class 2 Aggregate Base (inches)
Farm Equipment Pavements	7.0	3 4*	18 16

* Asphalt thickness includes Caltrans Factor of Safety.

In the summer heat, high axle loads coupled with shear stresses induced by sharply turning tire movements can lead to failure in asphalt concrete pavements. Therefore, we recommend that consideration be given to using the Portland cement concrete (PCC) section in areas subjected to concentrated heavy wheel loadings to reduce the potential for rutting of pavements. Based on the results of the field exploration and laboratory testing, we recommend a concrete paving section of six inches of Portland cement concrete over six inches of aggregate base compacted to at least 95 percent relative compaction.

We suggest that concrete slabs be constructed with thickened edges in accordance with American Concrete Institute (ACI) design standards. Reinforcing for crack control, if desired, should be determined by the project civil engineer or architect. Reinforcement must be located near mid-slab depth to be effective. Portland cement concrete should achieve a minimum compressive strength of 3500 pounds per square inch (psi) at 28 days. Concrete curing and joint spacing and details should conform with current Portland Cement Association (PCA) and ACI guidelines.

We emphasize that the performance of the pavement is dependent upon uniform and adequate compaction of the soil subgrade, as well as all engineered fill and utility trench backfill within the limits of the pavements. We recommend that pavement subgrade preparation (i.e.

scarification, moisture conditioning and compaction) be performed after underground utility construction is complete, and just prior to aggregate base placement. The upper six inches of pavement subgrade soils should be compacted to at least 95 percent of the ASTM D1557 maximum dry density at two percent above the optimum moisture content. Aggregate base should be compacted to at least 95 percent of the ASTM D1557 maximum dry density at two percent above the optimum moisture content. Materials quality and construction of the structural section of the pavements should conform to the applicable provisions of the Caltrans Standard Specifications, latest editions.

Site Drainage

Site drainage should be accomplished to provide positive drainage of surface water away from the buildings and prevent ponding of water adjacent to foundations. The subgrade adjacent to the building should be sloped away from foundations at a minimum two percent gradient for at least 10 feet, where possible. We recommend consideration be given to connecting all roof drains to solid PVC pipes which are connected to available drainage features to convey water away from the structures, or discharging the drains onto paved, or hard surfaces that slope away from the foundations.

Construction Observation Services

Site preparation should be accomplished in accordance with the recommendations of this report. Representatives of Wallace-Kuhl & Associates should be present during site preparation and all grading operations to observe and test the fill to verify compliance with our recommendations and the job specifications. These services are beyond the scope of work authorized for this investigation.

LIMITATIONS

Our recommendations are based upon the information provided regarding the proposed project, combined with our analysis of site conditions revealed by the field exploration and laboratory testing programs. We have used prudent engineering judgment based upon the information provided and the data generated from our investigation.

This report has been prepared in substantial compliance with generally accepted geotechnical engineering practices that exist in the area of the project at the time the report was prepared. No warranty, either express or implied, is provided.

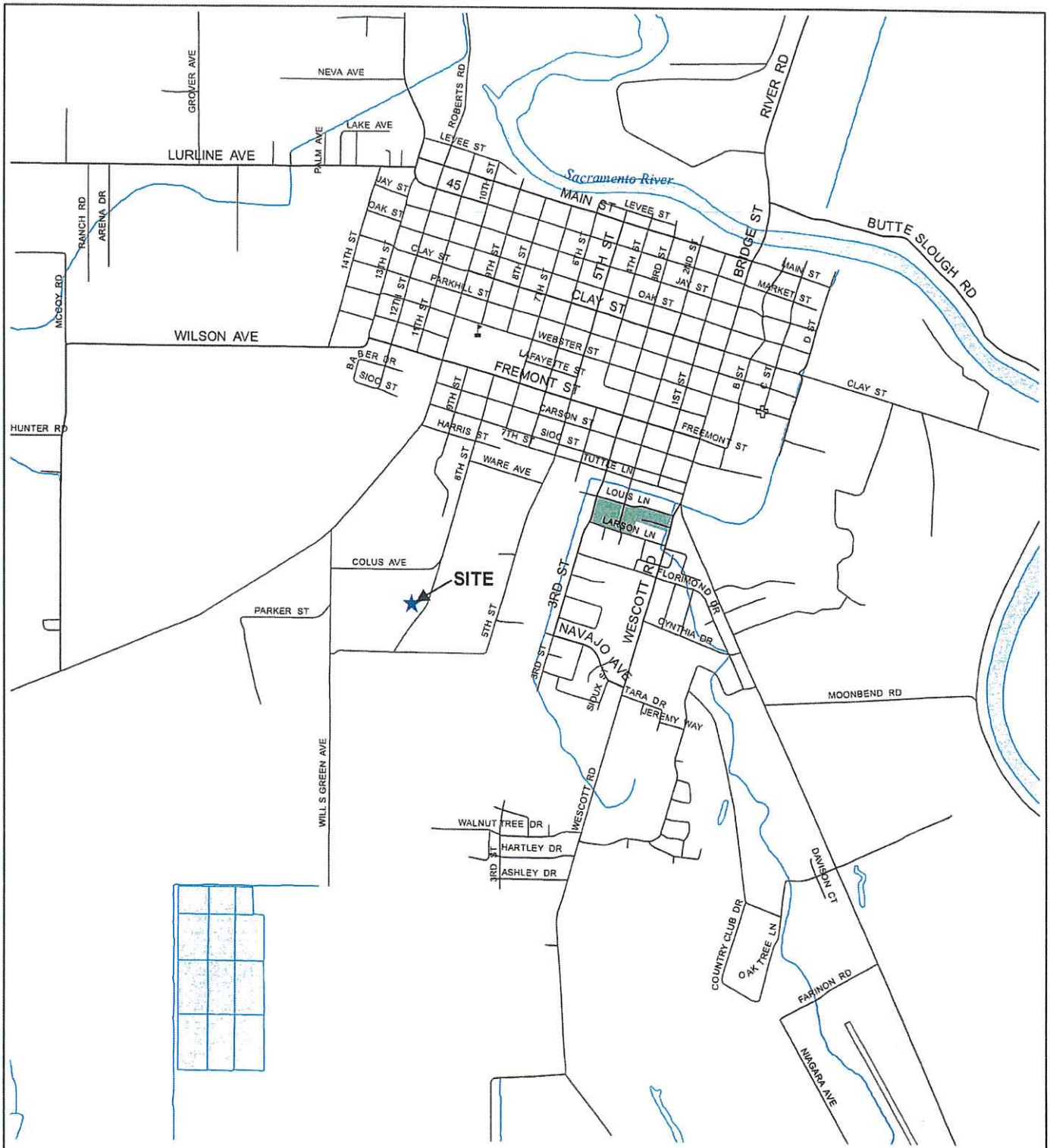
If the proposed construction is modified or re-sited; or, if it is found during construction that subsurface conditions differ from those we encountered at our boring locations, we should be afforded the opportunity to review the new information or changed conditions to determine if our conclusions and recommendations must be modified.

We emphasize that this report is applicable only to the proposed construction and the investigated site, and should not be utilized for construction on any other site.

The conclusions and recommendations of this report are considered valid for a period of three years. If design is not completed and construction has not started within three years of the date of this report, the report must be reviewed and updated if necessary.

Wallace - Kuhl & Associates

Matthew S. Moyneur
Senior Engineer



Street data courtesy of ESRI, 2007
Hydrography courtesy of the U.S. Geological Service
acquired from the GIS Data Depot, December, 2007
Projection: NAD 83, California State Plane, Zone II

WallaceKuhl & ASSOCIATES

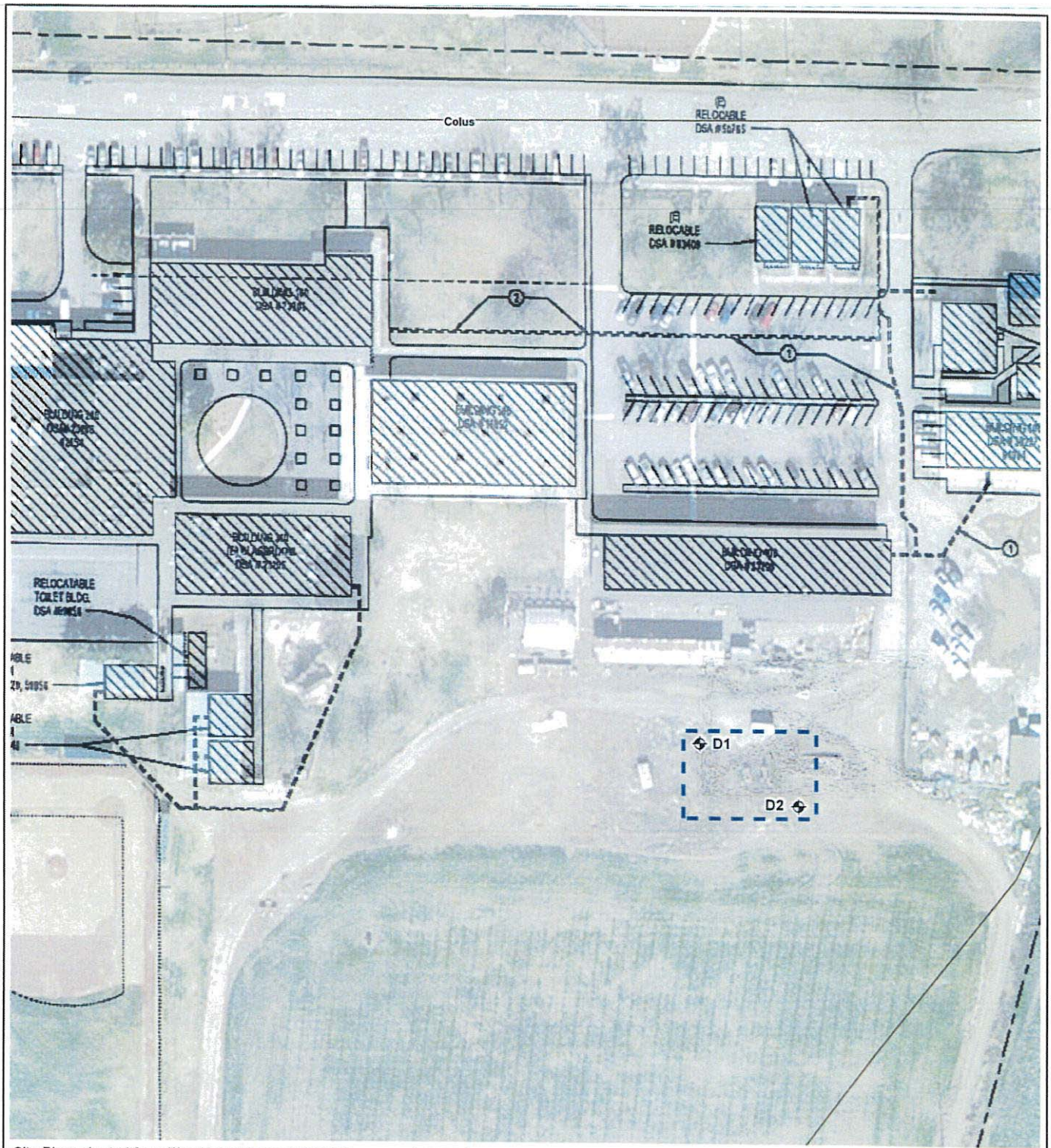
VICINITY MAP

COLUSA HIGH SCHOOL AG BARN BUILDING

Colusa, California

FIGURE	1
DRAWN BY	RWO
CHECKED BY	JDW
PROJECT MGR	MSM
DATE	12/15

WKA NO. 10796.01P



Site Plan adapted from Fire Alarm Plan provided by Architectural Nexus. Dated March 2015.
Aerial provided by ESRI.
Projection: NAD 83, California State Plane, Zone II

Legend

- Approximate Building Location
- Approximate Boring Location

N

0 50 100

Feet

WallaceKuhl & ASSOCIATES

SITE PLAN

COLUSA HIGH SCHOOL AG BARN BUILDING

Colusa, California

FIGURE	2
DRAWN BY	RWO
CHECKED BY	JDW
PROJECT MGR	MSM
DATE	12/15
WKA NO. 10796.01P	

Project: Colusa High School Ag Barn Building
Project Location: Colusa, California
WKA Number: 10796.01P

LOG OF SOIL BORING D1

Sheet 1 of 1

Date(s) Drilled: 12/7/15	Logged By: JDW	Checked By: MSM
Drilling Method: 4" Hand Auger	Drilling Contractor: Wallace Kuhl & Associates	Total Depth of Drill Hole: 9.0 feet
Drill Rig Type:	Diameter(s) of Hole, inches: 4"	Approx. Surface Elevation, ft MSL: 53.0
Groundwater Depth [Elevation], feet: Groundwater was not encountered [0.0]	Sampling Method(s): Open drive sampler with 6-inch sleeve	Drill Hole Backfill: soil cuttings
Remarks: Bulk sample (1 - 3'), EI		Driving Method and Drop: 10-lb slide hammer

ELEVATION, feet	DEPTH, feet	GRAPHIC LOG	ENGINEERING CLASSIFICATION AND DESCRIPTION	SAMPLE DATA: SAMPLE	SAMPLE DATA: SAMPLE NUMBER	SAMPLE DATA: NUMBER OF BLOWS	TEST DATA: MOISTURE CONTENT, %	TEST DATA: DRY UNIT WEIGHT, pcf	TEST DATA: ADDITIONAL TESTS
			Light brown, slightly moist, dense, silty fine sand with gravel, asphalt, and concrete (SM - FILL)						
			Brown, moist, clayey SILT (ML)	■	D1-1I				PI/GR
50									
	5		dark brown	■	D1-2I		26.5	87	
			Brown, moist, sandy, silty CLAY/clayey SILT (CL-ML)	■	D1-3I				PI/GR
45									
			End of hand auger boring at ~9 feet below ground surface. Groundwater was not encountered.						

BORING LOG 10796.01P - COLUSA HIGH SCHOOL AG BARN BUILDING.GPJ WKA.GDT 12/18/15 2:52 PM

Wallace Kuhl & Associates

FIGURE 3

Project: Colusa High School Ag Barn Building
Project Location: Colusa, California
WKA Number: 10796.01P

LOG OF SOIL BORING D2

Sheet 1 of 1

Date(s) Drilled: 12/7/15	Logged By: JDW	Checked By: MSM
Drilling Method: 4" Hand Auger	Drilling Contractor: Wallace Kuhl & Associates	Total Depth of Drill Hole: 9.0 feet
Drill Rig Type:	Diameter(s) of Hole, inches: 4"	Approx. Surface Elevation, ft MSL: 53.0
Groundwater Depth [Elevation], feet: Groundwater was not encountered [0.0]	Sampling Method(s): Open drive sampler with 6-inch sleeve	Drill Hole Backfill: soil cuttings
Remarks: Bulk sample (1 - 3')		Driving Method and Drop: 10-lb slide hammer

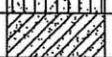
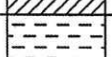
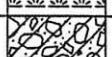
ELEVATION, feet	DEPTH, feet	GRAPHIC LOG	ENGINEERING CLASSIFICATION AND DESCRIPTION	SAMPLE DATA: SAMPLE	SAMPLE DATA: SAMPLE NUMBER	SAMPLE DATA: NUMBER OF BLOWS	TEST DATA: MOISTURE CONTENT, %	TEST DATA: DRY UNIT WEIGHT, pcf	TEST DATA: ADDITIONAL TESTS
			Light brown, slightly moist, dense, silty fine sand with gravel, asphalt, and concrete (SM - FILL)	■	D2-1I		18.8	103	
			Brown, moist, stiff, sandy, silty CLAY/clayey SILT (CL-ML)						
50				■	D2-2I		28.1	93	TR
	5								
				■	D2-3I				PI/GR
45									
				■	D2-4I		25.2	99	
			End of hand auger boring at ~9 feet below ground surface. Groundwater was not encountered.						

BORING LOG: 10796.01P - COLUSA HIGH SCHOOL AG BARN BUILDING.GPJ WKA.GDT 12/18/15 2:56 PM

Wallace Kuhl & Associates

FIGURE 4

UNIFIED SOIL CLASSIFICATION SYSTEM

MAJOR DIVISIONS		SYMBOL	CODE	TYPICAL NAMES
COARSE GRAINED SOILS (More than 50% of soil > no. 200 sieve size)	GRAVELS (More than 50% of coarse fraction > no. 4 sieve size)	GW		Well graded gravels or gravel - sand mixtures, little or no fines
		GP		Poorly graded gravels or gravel - sand mixtures, little or no fines
		GM		Silty gravels, gravel - sand - silt mixtures
		GC		Clayey gravels, gravel - sand - clay mixtures
	SANDS (50% or more of coarse fraction < no. 4 sieve size)	SW		Well graded sands or gravelly sands, little or no fines
		SP		Poorly graded sands or gravelly sands, little or no fines
		SM		Silty sands, sand - silt mixtures
		SC		Clayey sands, sand - clay mixtures
FINE GRAINED SOILS (50% or more of soil < no. 200 sieve size)	SILTS & CLAYS LL < 50	ML		Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity
		CL		Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
		OL		Organic silts and organic silty clays of low plasticity
	SILTS & CLAYS LL ≥ 50	MH		Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
		CH		Inorganic clays of high plasticity, fat clays
		OH		Organic clays of medium to high plasticity, organic silty clays, organic silts
HIGHLY ORGANIC SOILS		Pt		Peat and other highly organic soils
ROCK		RX		Rocks, weathered to fresh
FILL		FILL		Artificially placed fill material

OTHER SYMBOLS

= Drive Sample: 2-1/2" O.D. Modified California sampler

= Drive Sampler: no recovery

= SPT Sampler

= Initial Water Level

= Final Water Level

— — — = Estimated or gradational material change line

——— = Observed material change line

Laboratory Tests

PI = Plasticity Index

EI = Expansion Index

UCC = Unconfined Compression Test

TR = Triaxial Compression Test

GR = Gradational Analysis (Sieve)

K = Permeability Test

GRAIN SIZE CLASSIFICATION

CLASSIFICATION	RANGE OF GRAIN SIZES	
	U.S. Standard Sieve Size	Grain Size in Millimeters
BOULDERS	Above 12"	Above 305
COBBLES	12" to 3"	305 to 76.2
GRAVEL coarse (c) fine (f)	3" to No. 4 3" to 3/4" 3/4" to No. 4	76.2 to 4.76 76.2 to 19.1 19.1 to 4.76
SAND coarse (c) medium (m) fine (f)	No. 4 to No. 200 No. 4 to No. 10 No. 10 to No. 40 No. 40 to No. 200	4.76 to 0.074 4.76 to 2.00 2.00 to 0.420 0.420 to 0.074
SILT & CLAY	Below No. 200	Below 0.074

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UNIFIED SOIL CLASSIFICATION SYSTEM

COLUSA HIGH SCHOOL AG BARN BUILDING

Colusa, California

FIGURE	5
DRAWN BY	RWO
CHECKED BY	JDW
PROJECT MGR	MSM
DATE	12/15
WKA NO. 10796.01P	

APPENDIX A
General Project Information, Laboratory Testing and Results

APPENDIX A

A. GENERAL INFORMATION

The performance of a geotechnical engineering investigation for the proposed Colusa High School Ag Barn Building located at 901 Colus Avenue in Colusa, California, was authorized by Mr. Joseph Yee on December 2, 2015. Authorization was for an investigation as described in our proposal letter dated November 9, 2015, sent to our client Architectural Nexus, Inc., whose mailing address is 1990 Third Street, Suite 500 in Sacramento, California 95811; telephone (916) 443-5911.

In preparing this report we referenced an undated *Architectural Site Plan* prepared by Architectural Nexus, Inc.

B. FIELD EXPLORATION

Two hand auger borings were performed across the site on December 7, 2015, at the approximate locations indicated on Figure 2, utilizing hand auger equipment. The borings were extended to a depths of approximately nine feet below existing site grades using four-inch diameter hand augers. At various intervals, relatively undisturbed soil samples were recovered with a 2½-inch outside diameter (O.D.), 2-inch inside diameter (I.D.), core sampler driven manually by a 10-pound hammer.

The samples were retained in 2-inch diameter by 6-inch long thin-walled brass tubes contained within the sampler. Immediately after recovery the soils in the tubes were visually classified by the field engineer and the ends of the tubes were sealed to preserve the natural moisture contents. All samples were taken to our laboratory for additional soil classification and selection of samples for testing.

The Logs of Soil Borings, Figures 3 through 4, contain descriptions of the soils encountered at each boring location. A Boring Legend explaining the Unified Soil Classification System and the symbols used on the logs is contained on Figure 5.

C. LABORATORY TESTING

Selected undisturbed samples of the soils were tested to determine dry unit weight (ASTM D2937) and natural moisture content (ASTM D2216). The results of these tests are included on the boring logs at the depth each sample was obtained.

One relatively undisturbed sample was subjected to triaxial shear strength testing (ASTM D4767). The results of triaxial testing are presented on Figure A1.

Three samples of near-surface soil, considered to be representative of the on-site soils, were subjected to Plasticity Index testing (ASTM D4318). The test results are presented on Figure A2.

Three representative samples of near-surface soil were tested for grain-size distribution (ASTM C136) and hydrometer analysis (ASTM D422). The results of the gradation tests are contained on Figure A3.

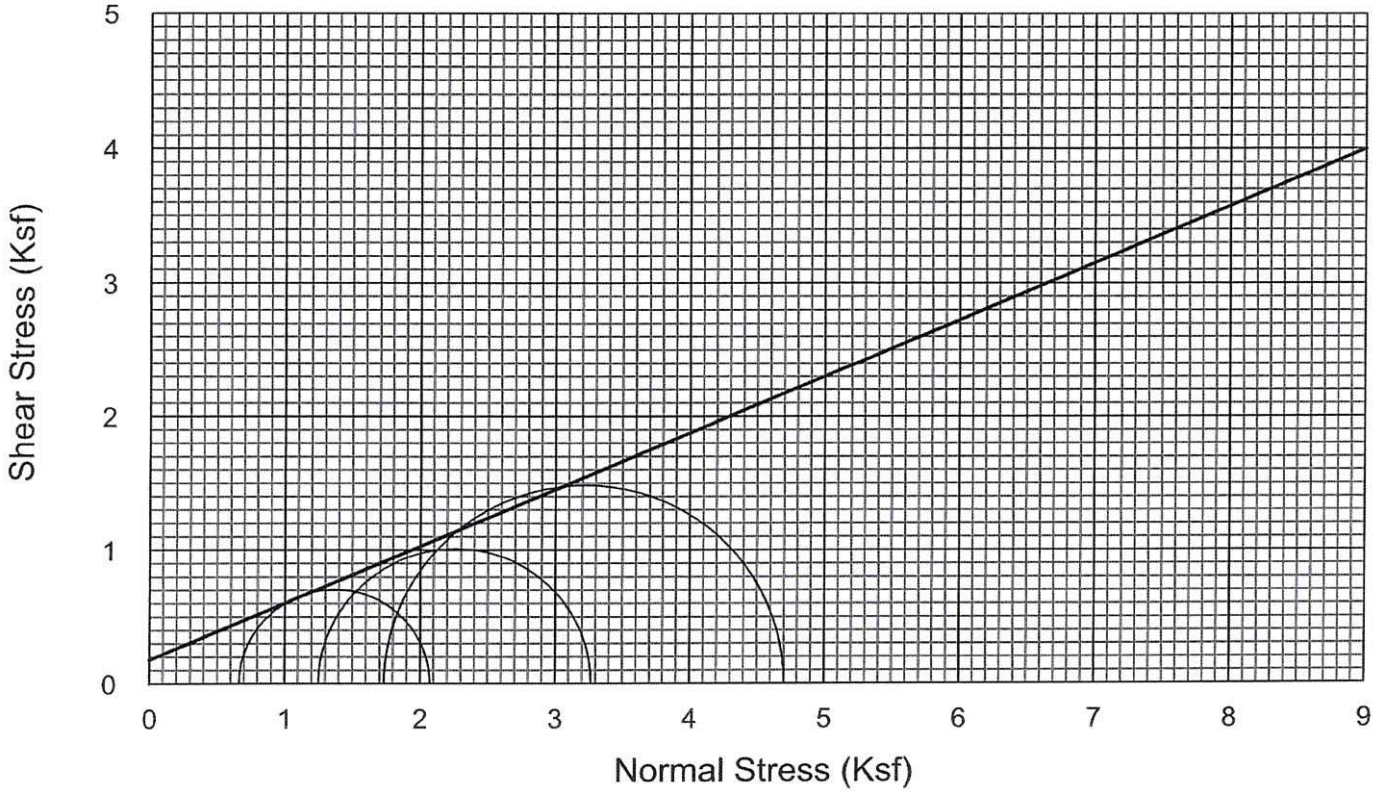
A bulk sample of near-surface soil was subjected to Expansion Index testing (ASTM D4829); the results of this test are presented on Figure A4.

A sample of the near-surface soil was submitted to Sunland Analytical to determine the soil pH and minimum resistivity (California Test 643), Sulfate concentration (California Test 417, ASTM D516) and Chloride concentration (California Test 422). The results of these tests are presented in Figure A5.

/

TRIAXIAL COMPRESSION TEST

ASTM D4767



SAMPLE NO. : D2-2I

SAMPLE CONDITION : Undisturbed

SAMPLE DESCRIPTION : Brown, silty clay/clayey silt

DRY DENSITY (PCF) : 93
INITIAL MOISTURE (%) : 28.1
FINAL MOISTURE (%) : 28.6

ANGLE OF INTERNAL FRICTION (ø) : 23°
COHESION (PSF) : 189

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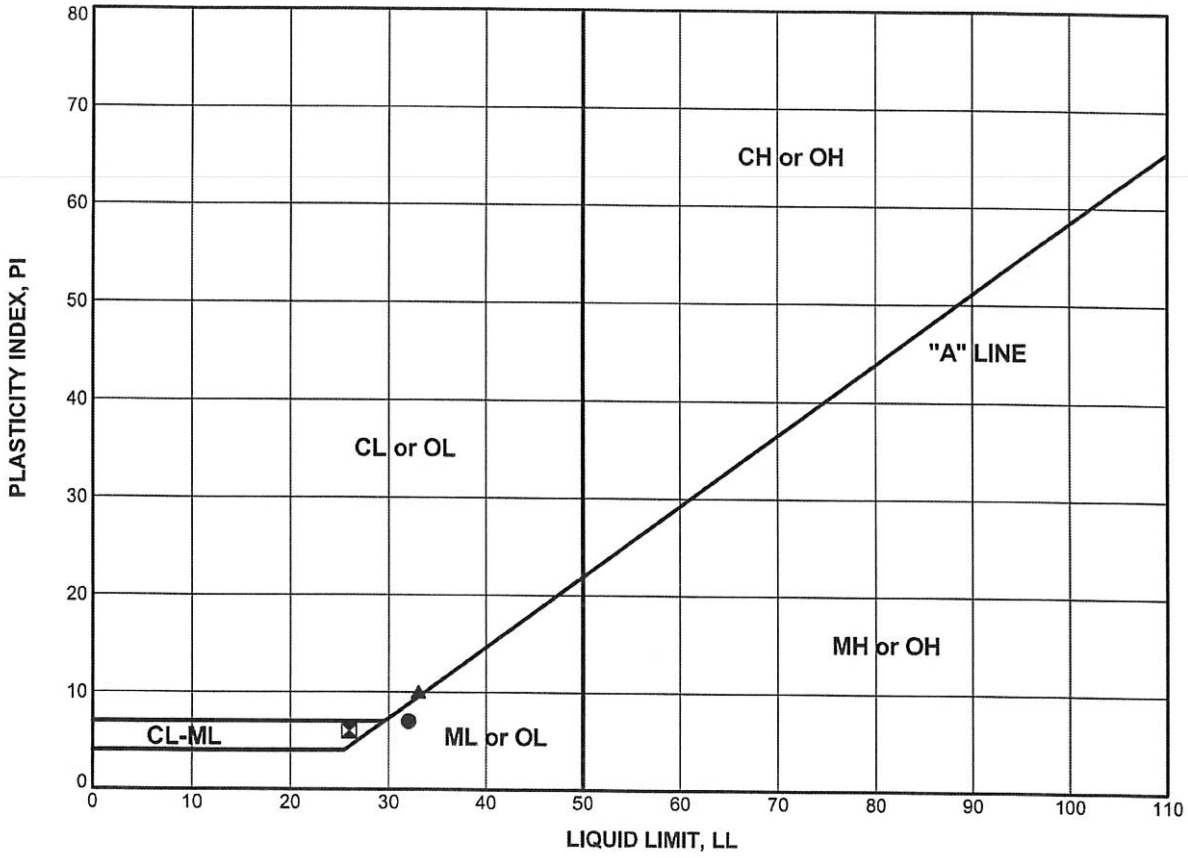
TRIAXIAL COMPRESSION TEST RESULTS

COLUSA HIGH SCHOOL AG BARN BUILDING

Colusa, California

FIGURE	A1
DRAWN BY	RWO
CHECKED BY	JDW
PROJECT MGR	MSM
DATE	12/15

WKA NO.10796.01P



Boring Number	Sample Number	USCS	Depth (feet)	Test Symbol	Water Content (%)	LL	PL	PI	Classification
D1	D1-1I	ML	1.5 - 2.0	●	-	32	25	7	brown, clayey silt
D1	D1-3I	CL-ML	7.5 - 8.0	⊠	-	26	20	6	brown, sandy, silty clay/clayey silt
D2	D2-3I	CL-ML	6.0 - 6.5	▲	-	33	23	10	brown, silty clay/clayey silt

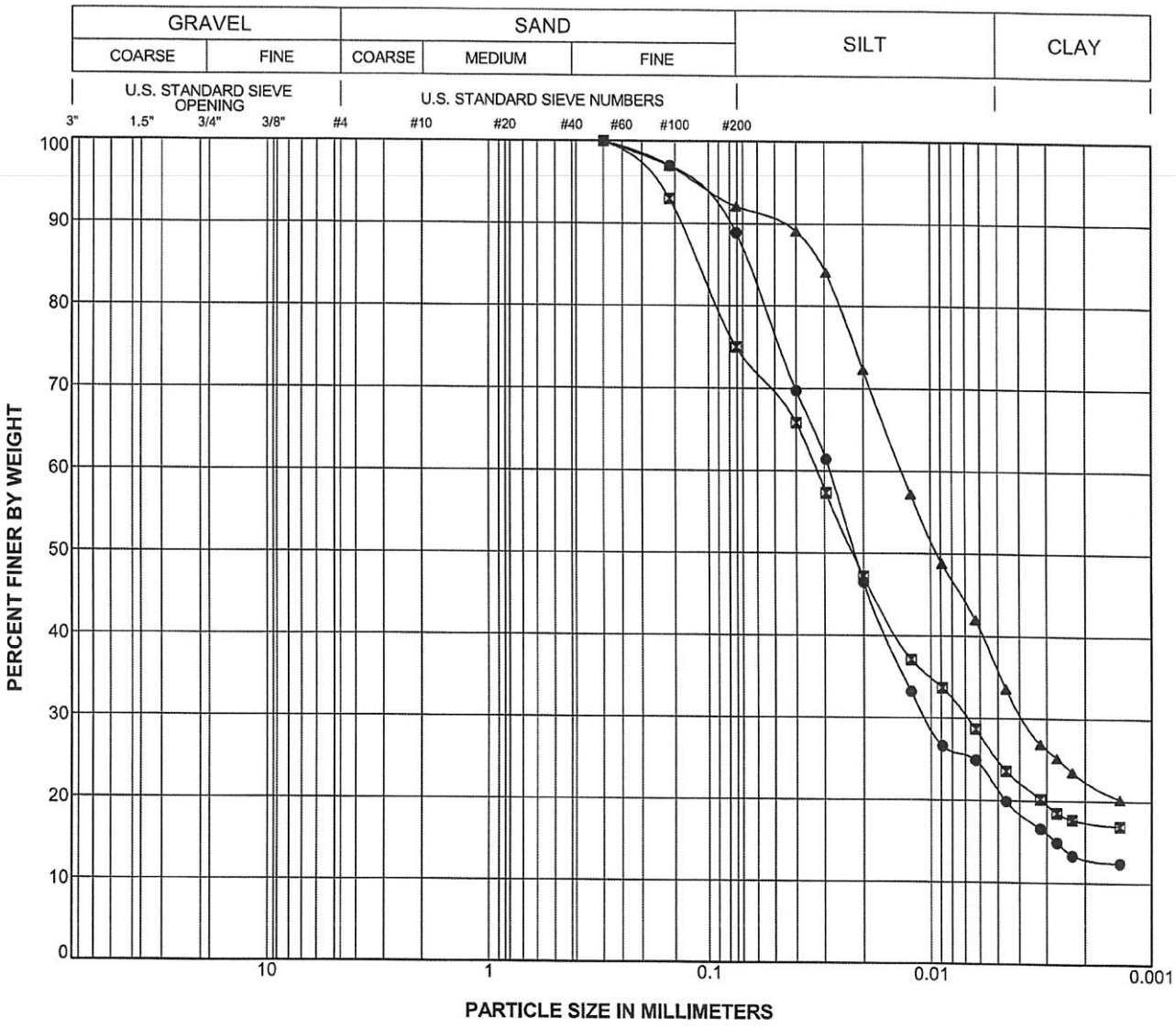
ATTERBERG LIMITS 12 PTS 10796.01P - COLUSA HIGH SCHOOL AG BARN BUILDING.GPJ WKA.GDT 12/16/15 1:40 PM

ATTERBERG LIMITS (ASTM D4318)

Project: Colusa High School Ag Barn Building
WKA No. 10796.01P

FIGURE A2

Wallace Kuhl & Associates



Boring Number	Sample Number	USCS	Depth (feet)	Symbol	LL	PI	Classification
D1	D1-1I	ML	1.5 - 2.0	●	32	7	brown, clayey silt
D1	D1-3I	CL-ML	7.5 - 8.0	⊠	26	6	brown, sandy, silty clay/clayey silt
D2	D2-3I	CL-ML	6.0 - 6.5	▲	33	10	brown, silty clay/clayey silt

PARTICLE SIZE DISTRIBUTION

Project: Colusa High School Ag Barn Building
WKA No. 10796.01P

FIGURE A3

Wallace Kuhl & Associates

GRAIN SIZE 10796.01P - COLUSA HIGH SCHOOL AG BARN BUILDING.GPJ WKA.GDT 12/16/15 1:43 PM

EXPANSION INDEX TEST RESULTS

ASTM D4829

MATERIAL DESCRIPTION: Brown, clayey silt with sand

LOCATION: D1

Sample Depth	Pre-Test Moisture (%)	Post-Test Moisture (%)	Dry Density (pcf)	Expansion Index
1 - 3'	11.9	23.5	103.8	**51**

CLASSIFICATION OF EXPANSIVE SOIL *

EXPANSION INDEX	POTENTIAL EXPANSION
0 - 20	Very Low
21 - 50	Low
51 - 90	**Medium**
91 - 130	High
Above 130	Very High

* From ASTM D4829, Table 1

WallaceKuhl & ASSOCIATES

EXPANSION INDEX

COLUSA HIGH SCHOOL AG BARN BUILDING

Colusa, California

FIGURE	A4
DRAWN BY	RWO
CHECKED BY	JDW
PROJECT MGR	SLF
DATE	12/15
WKA NO. 10796.01P	

Sunland Analytical
11419 Sunrise Gold Circle, #10
Rancho Cordova, CA 95742
(916) 852-8557

Date Reported 12/11/2015
Date Submitted 12/08/2015

To: Joseph Waltz
Wallace-Kuhl & Assoc.
3050 Industrial Blvd
West Sacramento, CA 95691

From: Gene Oliphant, Ph.D. \ Randy Horney
General Manager \ Lab Manager

The reported analysis was requested for the following location:
Location : 10796.01P Site ID : BULK D2@1-3FT.
Thank you for your business.

* For future reference to this analysis please use SUN # 70956-148033.

EVALUATION FOR SOIL CORROSION

Soil pH	7.97		
Minimum Resistivity	1.23 ohm-cm (x1000)		
Chloride	68.3 ppm	0.00683	%
Sulfate	72.1ppm	0.00721	%

METHODS
pH and Min.Resistivity CA DOT Test #643 Mod.(Sm.Cell)
Sulfate CA DOT Test #417, Chloride CA DOT Test #422

Extractable Sulfate in Water

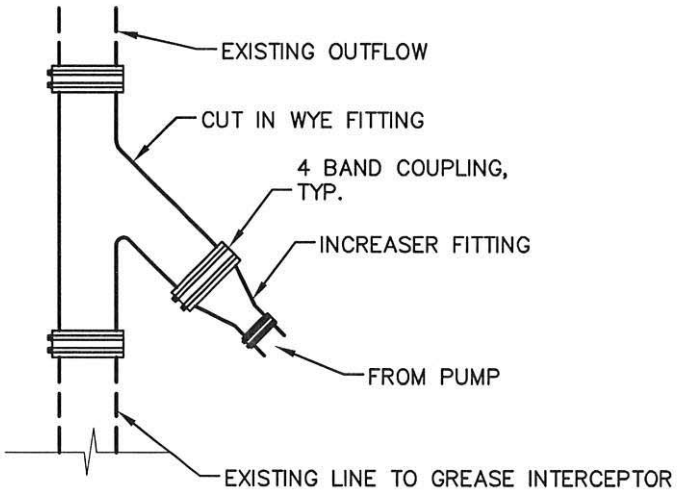
TYPE OF TEST	RESULTS	UNITS
Sulfate-SO4	63.65	mg/kg

ASTM D-516 from sat.paste extract-reported based on dry wt.

WallaceKuhl & ASSOCIATES

CORROSION TEST RESULTS
COLUSA HIGH SCHOOL AG BARN BUILDING
Colusa, California

FIGURE	A5
DRAWN BY	RWO
CHECKED BY	JDW
PROJECT MGR	MSM
DATE	12/15
WKA NO. 10796.01P	



4 / C1.1 SEWER CONNECTION

NO SCALE

WCE
1117 WINDFIELD WAY, SUITE 110
EL DORADO HILLS, CA 95762
PH (916) 985-1870
FAX (916) 985-1877

2915-197 C11A

ARCH | NEXUS

PROJECT

UTILITY PLAN

COLUSA UNIFIED SCHOOL DISTRICT
COLUSA HS AG BARN SITE UTILITIES

901 COLUSA AVENUE, COLUSA, CA

DOCUMENT TYPE

APPLICATION

SHEET

DATE
03/03/2016
JOB

REFERENCE SHEET
C1.1

Colusa Unified School District
Bid # 16-100
Colusa High School Ag Barn Site Utilities Project
Colusa High School
901 Colus Avenue, Colusa, CA 95843

PRE-BID CONFERENCE AGENDA

Date: Thursday, February 25, 2016 **Time:** 3:00pm

Project: Bid Package: #16-100 Colusa High School Ag Barn Site Utilities Project - Colusa HS
Bid Date: **Thursday, March 10, 2016 at 2:00 p.m.** (see Section 00 010 Notice to Bidders for location)

I. **Meeting Called to Order**
II. **Introduction of Project Team members:**
   A. Owner's Representative(s)
   B. Capital Program Management
III. **Bidding Documents:** Available from Signature Reprographics, Sacramento, 916-454-0800
IV. **Contracting Format:** Prime Contract
V. **Scope of Work Descriptions:** Section 00 8000, Article 1
VI. **Project Budget:** $32,775
VII. **Bidding and Contract Award Requirements:**
   A. License requirement(s): Class A or B
   B. Bid Bond or Certified Check amount for 10% of the bid amount
   C. Prevailing Wages - Section 00 7200, Article 15. Certified payrolls, payroll records and other documents shall be required along with your progress billings. **www.dir.ca.gov/dlsr/DPreWageDetermination.htm.**
   D. DIR Registration of Contractor and Subcontractor: Notice to Bidders Section 00 1116 and Instructions to Bidders Section 00 2113.
   E. Bond (Section 00 7200, Article 10) and Insurance Requirements (Section 00 7200, Article 25)
   F. Proposal Form:
      1. Completed Forms
      2. No exclusions
      3. No faxes or phone bids
      4. Bids good for 90 days
IX. **Inspection Procedures:** TBD
X. **Project Schedule:** Section 00 8000, Article 3
XI. **Department of Justice (DOJ) Clearance, Badges and Security:** Section 00 7200, Article 51
XII. **Site Information**:
   A School contacts: Principal, Darren Brown, Office Contact 530-458-2156.
   B. Site access, temporary facilities, staging areas and parking
   C. Conduct on school premises
   D. Contractor's working hours
   E. Contractor's supervision
XIII. **Site Walk**
XIV. **Questions**
XV. **Adjournment**

Important note: Responses to inquiries and discussions occurring at this pre-bid walk-through shall in no way change or modify the bid documents. The bid documents will be affected only by addenda issued prior to the bid date.

Send inquiries to: wally@capitalpm.com
Capital Program Management, Inc.
1851 Heritage Lane, Suite 210, Sacramento, CA 95815

Colusa Unified School District

PRE-BID CONFERENCE:	**Colusa High School Ag Barn**
DATE: February 25th, 2016	**Site Utilities Project**
TIME: 3:00 p.m.	**LOCATION: Colusa High School**
BID NO: #16-100	**BID OPENS: March 10th, 2016 at 2:00 p.m.**

SIGN-IN SHEET
(Please Print)

COMPANY NAME: Fletchers Plumbing LICENSE TYPE A B C36

ADDRESS: 219 Burns Dr. Yuba City Ca 95991

PHONE: (530) 673-2489 FAX: (530) 673-1317

REPRESENTATIVE: Brian Madison E-MAIL bm.6@fpci.us

HOW DID YOU HEAR ABOUT THIS PROJECT? Terry Bladen email

COMPANY NAME: Schmidt Construction Inc LICENSE TYPE A, B, C8

ADDRESS: PO BOX 95, Maxwell, CA 95955

PHONE: (530) 438-2235 FAX: (530) 438-2298

REPRESENTATIVE: Amy Gwinnup E-MAIL amy@schmidtconst.com

HOW DID YOU HEAR ABOUT THIS PROJECT? CPM

COMPANY NAME: ABS Builders, Inc LICENSE TYPE ABC-8

ADDRESS: PO BOX 95 Maxwell, CA 95955

PHONE: 530 438-2235 FAX: 530 438-2298

REPRESENTATIVE: Amy Gwinnup E-MAIL amy@absbuildersinc.com

HOW DID YOU HEAR ABOUT THIS PROJECT? Schmidt

---

5 MIN LATE

COMPANY NAME: REM CONSTRUCTION LICENSE TYPE A-B-C-8

ADDRESS: 5788 BONNIE LN PARADISE

PHONE: (530) 876-1363 FAX: (530) 876-1169

REPRESENTATIVE: DAN SCHELL E-MAIL DANIEL-REM@SBCGLOBAL.NET

HOW DID YOU HEAR ABOUT THIS PROJECT? VCE

ADDENDUM NO. _____